



The Open Civil Engineering Journal

Content list available at: www.benthamopen.com/TOCIEJ/

DOI: 10.2174/1874149501711010303



RESEARCH ARTICLE

Mechanical Properties Research on Suspended-dome Structure with Discontinuous Support Under Different Parameters

Yu Jinghai^{1,2,*}, Leng Ming¹, Zhang Zhongyu¹, Jiang Zhiyu¹ and Wang Zhengkai³

¹School of Civil Engineering, Tianjin University, Tianjin, China

²Architectural Design and Research Institute of Tianjin University, Tianjin, China

³Tianjin Construction Engineering Group Real Estate Development Co., LTD., Tianjin, China

Received: June 26, 2016

Revised: February 07, 2017

Accepted: February 08, 2017

Abstract:

Introduction:

For suspended-dome with big partial hole, two kinds of feasible plans were proposed.

Method:

One of the plans was to adjust suspended-dome's members, and another to build auxiliary supporting system to support the suspended-dome with columns together. These kinds of suspended-dome's main structural parameters include stiffness, member force, steel consumption, support reaction and stability, and parametric analysis is based on these parameters.

Conclusion:

Firstly, the scope of application was illustrated for the former plan. Then, an equivalent vertical stiffness algorithm was put forward for the latter plan. In the latter plan, limit stiffness constant was introduced to quantify the method for solving discontinuous support problem.

Keywords: Suspended-dome, Discontinuous Support, Parameterization, Equivalent Stiffness, Parameters, Single-layer shells.

1. INTRODUCTION

Suspended-dome was first put forward by Prof. Kawaguchi, A. Masaru, I. Tatemichi, *et al* [1]. As a kind of large-span space structure, suspended-dome structure combines advantages of cable dome with single-layer shell. The suspend-dome system is composed of single-layer shells, prestressing cable and strut. The upper end of the strut is hinged to the single-layer reticulated shell and lower end of it is connected together by hoop which forms a complete closed force system. Under the action of external force, the thrust of the upper single-layer shell is reduced due to the pretension of cable. The internal force is firstly transmitted from single-layer shell to strut, then from strut to cable, and finally from cable to support [1]. Currently, suspended-dome has been widely used all over the world. As a kind of self-balanced spatial structure architecture, one important reason why suspended-dome is widely welcomed by designers is that it has an explicit force transfer path and excellent mechanical properties. Another reason is that it reduces the amount of steel used in large span structure [1 - 3]. Somehow this structure's unique characteristics determine that its shape should meet some basic requirements, such as regularity, integrity and symmetry, etc. Due to the architecture functional requirements, some public buildings such as stadium, concert hall and auditorium that use suspended-dome structure usually need big partial holes. And because of this, it is difficult to ensure that the position of the supporting

* Address correspondence to this author at Room 1004, No. 192, anshan west road nankai district, Tianjin, China; Tel: +86-22-27401393; Fax: +86-15-222182802; E-mails: yjh300072@163.com, lm41000@163.com

column can be uniformly and continuously arranged under the roof structure. Consequently, reasonable solutions for suspended-dome structure with discontinuous supporting have great practical significance.

2. RESEARCH ON FEASIBILITY SOLUTIONS

Two kinds of schemes, with or without assistant support, were proposed for suspended-dome with discontinuous support. The schematic diagram of the two feasible schemes is shown in Fig. (1).

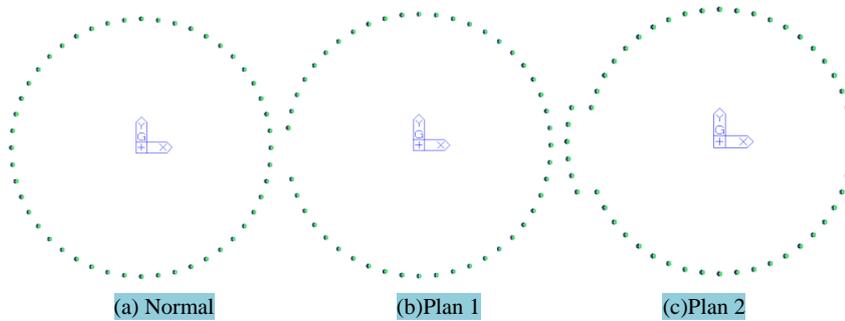


Fig. (1). Schematics of support arrangement plan.

Plan 1, parametric analysis without assistant support: The substructure provides a single circle concrete column as the supporting system. The part which lacks column cannot form an assistant supporting structure, and this problem only can be solved by adjusting members' specification of the suspended-dome.

Plan 2, parametric analysis with assistant support: In addition to the substructure, a single circle concrete column is provided as the supporting system, and the missing columns' part of the structure can be replaced by the assistant supporting system. This system can support the suspended-dome with the external concrete column together.

Circular Kiewitt suspended-domes of 40m, 80m and 120m span were selected as examples to be applied to the two kinds of schemes and corresponding parametric analysis plans.

3. PARAMETRIC ANALYSIS OF NONE ASSISTANT SUPPORT PLAN

In this section, parametric analysis plan was established for two most important parameters: the number of missing columns and the span. The applicability of the plan was determined by the change rule of all parameters.

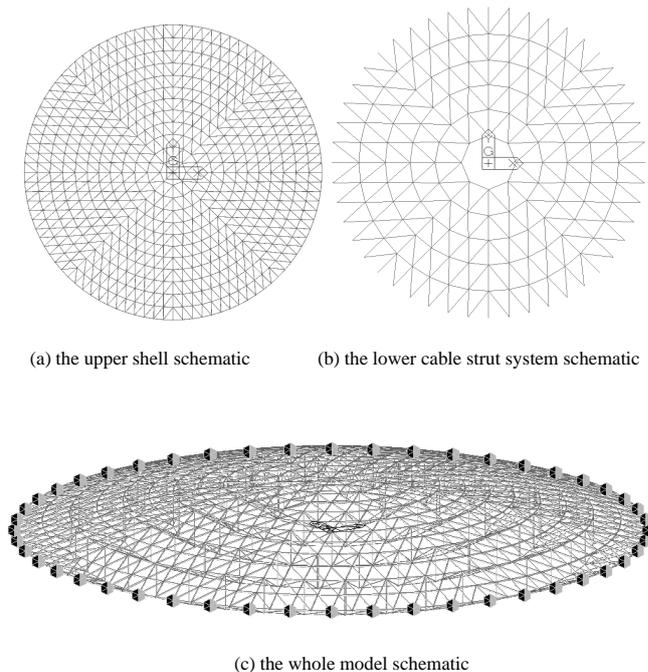


Fig. (2). The basic analysis model.

As for the two plans above, we chose standard circular Kiewitt suspended-dome structure to conduct the analysis and selected parameters of span, span ratio and number of missing columns for research. The shell part of the structure was K-8 type, and the lower cable strut system was in the form of a thin cable with 5 rings, and the support was made of the three direction hinged support. We used Midas Gen software [4, 5] to establish the model and conduct the analysis. In the model, beam element was simulated for rod element in shell, truss element for the compression strut, tension-only element for cable and pull rod. The basic analysis model is shown above Fig. (2).

3.1. Parametric Analysis Condition

In most suspended-dome structures, the interval between columns is generally between 6 and 8 m. The number of missing columns is the most direct parameter. Therefore, the number of missing columns can basically reflect the span of the structure of the missing area [6].

The parameters of the suspended-dome structure are set as shown below. Its planar projection is the standard circular. The K-8 type is selected as the net shell part form, and the 3m-long rods are selected to control the grid size. The lower cable-strut system is formed for thin rope type. The rise-span ratio is 1/12, and the height of poles is 4.5 m. The number of missing columns is between 1 and 4, the main spans are 40 m, 80 m and 120 m.

Midas Gen was used to establish the model and analyze the static as well as dynamic performance, while ABAQUS was used to analyze stability of the structure [7]. The maximum members, the maximum displacement, the maximum support reactions, the amount of steel and the stability of the structure were selected as the comparison parameters. Maximum members reflected the maximum stress, maximum displacement reflected the overall stiffness of the structure, maximum support reactions reflected the feasibility of structure, and the amount of steel reflected the rationality of the structure, respectively. The failure mode and the load factor-displacement curve can be obtained through stability analysis. Through considering the five parameters mentioned formerly, the rationality of the structure can be proved.

(I) Parametric analysis results of suspended-dome of 40m span

The significance of the parameters which are not shown in the following table has the same meaning in this table.

As shows in the Table 1, for the suspended-dome with 40m span, the number of missing columns only affected the selection of rod specification and specification of rod was always controlled by the slenderness ratio. With the increase of number of missing columns, the displacement showed a consistent trend, but the displacement was not the right controlling factor. The maximum support reaction's increment was more noticeable than the displacement. When the number of missing column hatted 3, the maximum support reaction had reached 10 times as much as a normal condition. The amount of steel was less affected by the number of missing columns. From the stability, the lack of columns would cause local vibration. According to load factor-displacement curves (Fig. 3), when the number of missing column was 1 or 2, the limit load factor change fewer, and the displacement matched by the ultimate load increased little. Nonetheless, there was a decrease of slope in the climbing part. When the number of missing column was up to 3, the superme load factor reached 1.95, which caused a falling part, not meeting the design needs. Above all, for the suspended-dome of 40m span, the missing number of columns was limited to 2 [8 - 11].

Table 1. Comparison of suspended-dome of 40m span between different missing conditions.

Comparison of Parameters	Numbers of Missing Column				
	none	1	2	3	4
Maximum member specifications	Φ180×8	Φ180×8	Φ180×8	Φ245×12	Φ245×12
Displacement, mm	6	20	32	76	74
Maximum support reactions, kN	86	171(1.98)	300(3.49)	910(10.58)	1297(15.1)
Amount of steel, kg/m ²	61.55	61.55	61.55	62.85	62.9
Limit load factors	10.81	10.58	10.39	1.96	—

Note: Data in brackets are the ratio of the current data to the normal. Maximum member specifications in bracket mean the maximum rod specifications of suspended-dome. Signal Φ180×8 means 180mm diameter and 8mm thickness of the steel tube, and other symbols represent the same meaning. Displacement shows the maximum displacement of the structure under the action of load D+L+P (transverse load + live load + prestressing load). Maximum support reactions represent the maximum horizontal force acting under the load above. Amount of steel represents the amount of steel used per square meter. Limit load factors mean ultimate load factor, which is used to reflect the deviation of the design load and the actual load.

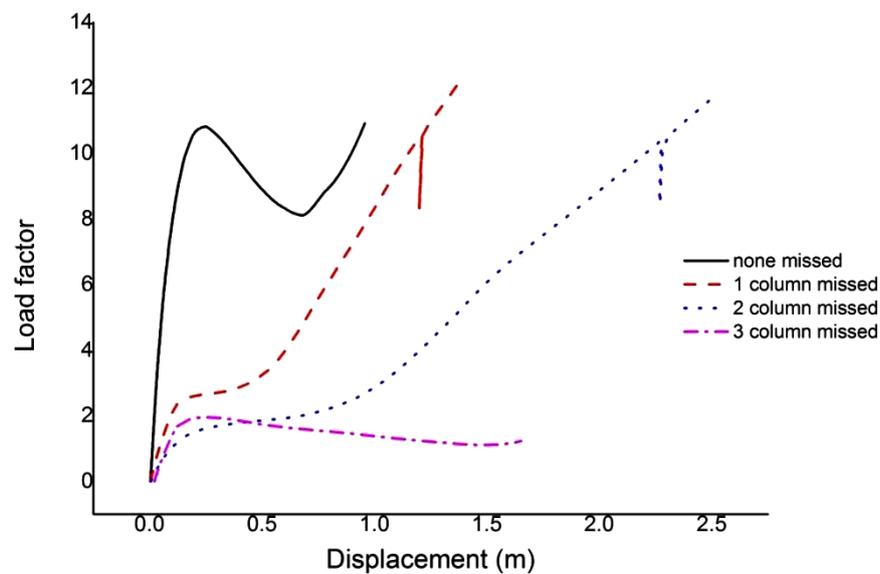


Fig. (3). Suspended-dome of 40m span load factor-displacement curves.

(II) Parametric analysis results of suspended-dome of 80m span

As shown in Table 2, for the suspended-dome of 80m span, when the number of missing columns was more than 2, the members stress showed a significant change, but the amount of steel changed slightly. The displacement was extended with the increase of the number of missing columns. When the number of missing columns was up to 4, the reaction force was 9 times higher than the normal condition. The stability could consist of three stages with the increase of the number of missing columns. On the first stage, when the number of missing columns was smaller, the stability was affected slightly, with a little decrease of the overall stiffness. As is shown in Fig. (4), when the number of missing column was 1, the failure mode was similar to the normal condition.

Table 2. Comparison of suspended-dome of 80m span between different missing conditions.

Comparison of Parameters	Numbers of Missing Column				
	none	1	2	3	4
Maximum member specifications	Φ180×8	Φ219×10	Φ219×10	Φ402×14	Φ500×16
Displacement, mm	48	49	66	101	110
Maximum support reactions, kN	283	438 (1.55)	615 (2.17)	1897 (6.70)	2637 (9.31)
Amount of steel, kg/m ²	64.27	64.27	64.31	64.8	65.13
Limit load factors	3.8	3.79	3.78	3.42	3.15

Note: Data in brackets is the ratio of the current data to the normal.

On the second stage, the missing column started to affect the overall stability. The local vibration mode occurred but the feature value was a little bit larger. Besides, the ductility decreased, which was reflected in the material-geometry double nonlinear. According to Fig. (4), when the number of missing column was 2, the failure mode was shown as the dramatic increase of the whole and the local displacement, accompanied by excessive deformation. Then, the properties of the most outer ring cable declined sharply, until the structure damaged.

On the third stage, part missing of columns led a profound impact on the overall stability of the structure. In the process of buckling analysis, it appeared local vibration mode which was more advanced than normal condition. In this process, limit load factors decreased and integral curve showed the same trend as the load coefficient- displacement curve of single layer reticulated shell. And these performances show that effect of the cable strut system could be reduced significantly by a lack of excessive columns. Just as shows in the load factor- displacement curve in Fig. (4), when the structure had a lack of 3 or 4 columns, the failure mode of the structure was indicated by the dramatic increase of the displacement in part of the structure. The local member failure first, then the other members which near the damaged member failure which caused the area of lacking columns failure, and then the outer ring became invalid

because of large deformation, finally the whole structure damage. In summary, as for the suspended-dome of 80m span (Fig. 5), the missing number of columns was limited to 2.

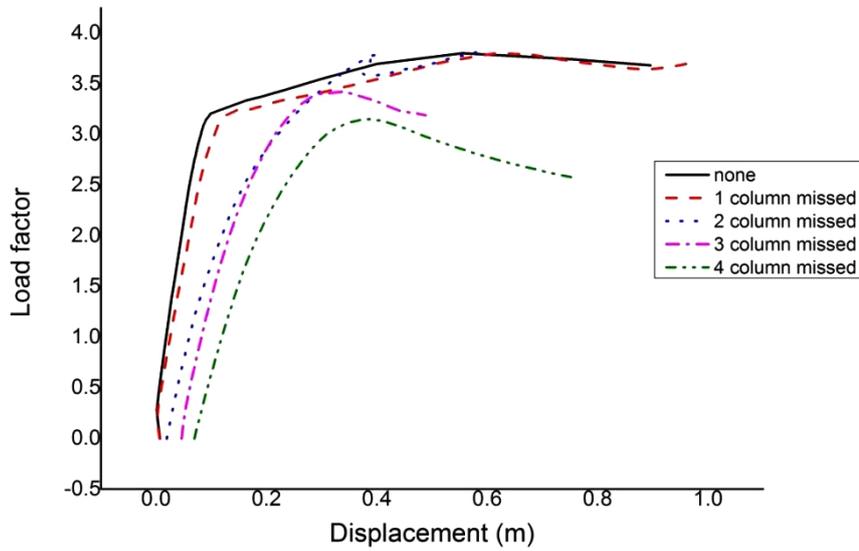


Fig. (4). Suspended-dome of 80m span load factor-displacement curves.

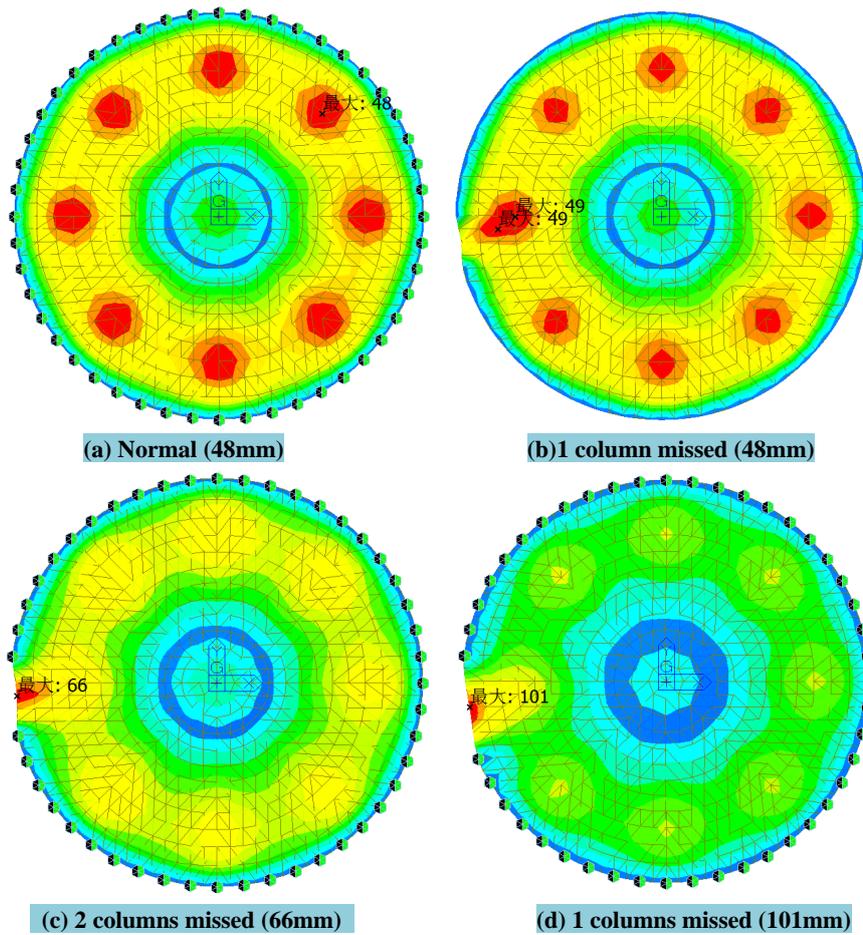


Fig. (5). Displacement of suspended-dome of 80m span.

(III) Parametric analysis results of suspended-dome of 120m span

As shown in the Table 3 and Figs. (6 and 7), for the suspended-dome of 120m span, when the number of missing columns was 1, the effect on members of stress was significant, but the affected area was small. For stability, when the structure was a shortage of 2 columns, the mode was still dominated by the whole vibration and the first 50 vibration modes of the structure did not show local vibration. Therefore, we were able to get the load factor-displacement curves above. The maximum value of the support reaction increased with the growth of the number of missing columns. When the structure lacked 3 columns, its maximum member specification must be $\Phi 630 \times 16$ or more, which exceeded the limit, so the number of missing columns were limited to 2 [8 - 11].

Table 3. Comparison of suspended-dome of 120m span between different missing conditions.

Comparison of Parameters	Numbers of Missing Column			
	none	1	2	3
Maximum member specifications	$\Phi 245 \times 12$	$\Phi 245 \times 12$	$\Phi 500 \times 16$	$> \Phi 630 \times 16$
Displacement, mm	131	131	131	—
Maximum support reactions, kN	569	803 (1.41)	1666 (2.93)	—
Amount of steel, kg/m ²	86.57	86.57	86.98	—
Limit load factors	4.43	4.42	4.42	—

Note: Data in brackets is the ratio of the current data to the normal.

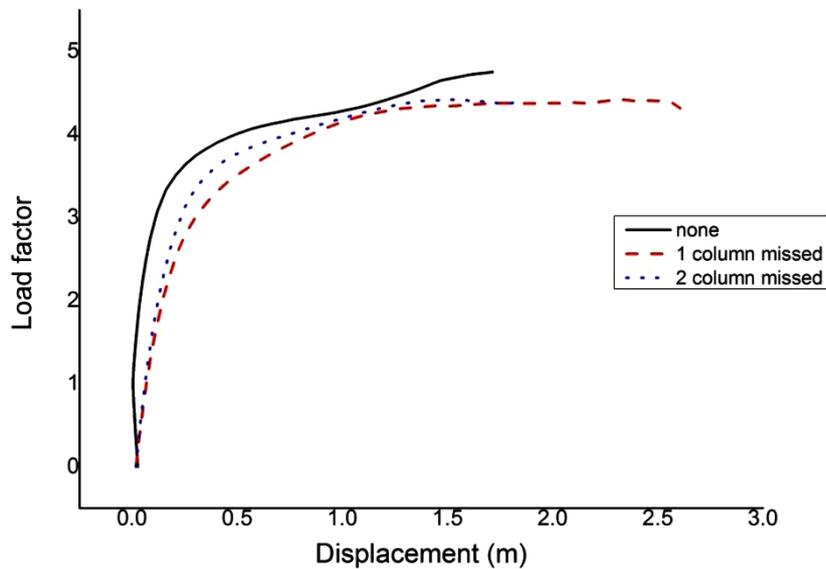


Fig. (6). suspended-dome of 120m span load factor-displacement curves.

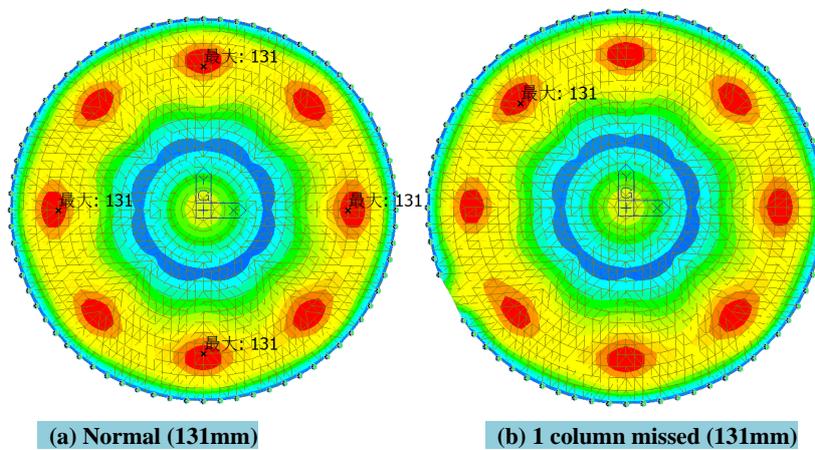


Fig. (7). displacement of suspended-dome of 120m span.

3.2. Parametric Analysis of Suspended-dome Under Various Span

The limit value of plan 1 and the comparison results between normal and the model under its respective limits are listed in Table 4.

Table 4. Limit missing columns and results under different span.

Span, m	Numbers of Missing Column	Maximum Members	Displacement, mm	Maximum Support Reactions, kN	Amount of Steel, kg/m ²	Limit Load Factors
40m	0	Φ180×8	6	86	61.55	10.81
	2	Φ180×8	32	300 (3.49)	61.55 (1.000)	10.39
53.3m	0	Φ180×8	17	137	62.34	8.83
	2	Φ325×12	89	1162 (8.48)	62.84 (1.008)	8.52
66.6m	0	Φ180×8	48	283	92.75	6.78
	2	Φ325×12	82	1380 (4.88)	93.26 (1.005)	6.52
80m	0	Φ180×8	48	283	64.27	3.8
	3	Φ402×14	101	1597 (5.64)	64.80 (1.008)	3.42
93.3m	0	Φ180×8	84	352	64.98	3.24
	3	Φ560×16	84	2165 (6.15)	66.73 (1.027)	3.22
106.6m	0	Φ219×10	100	518	87.98	4.24
	2	Φ450×14	97	1653 (3.19)	88.68 (1.008)	4.25
120m	0	Φ245×12	131	569	86.57	4.43
	2	Φ500×16	131	1666 (2.93)	86.98 (1.005)	4.42

After analyzed the data in Table 4, we found that the data show the following regularities under different spans.

(1) With the increase of the span and the number of missing columns, maximum stress increases faster, which mainly centered on the ring bar. Lack of columns only affects the stress of the partial members of missing area, as for steel consumption of this structure, the effects can be ignored.

(2) With the number of missing columns rise, local displacement gradually increases. When the maximum displacement presents in the position of missing area, the displacement increases significantly when the number of missing columns grows. Support reaction has a significant change with the number of missing columns increase. It should be pointed out that the support reactions of the edge columns are one of the controlling reasons of whether the structure is feasible.

(3) The lack of column has a complex influence on the structural stability. According to the local stiffness in the area of missing column, stability of the structure has a different performance. In some analysis model, a phenomenon occurred that the stiffness of the column is more than that of the original structure and the limit load coefficient slightly increase as the local bar specifications become larger. While in general, the limit load factor decreases with the increase of span.

In a word, when the number of missing columns is not more than 2, this plan is much more reasonable.

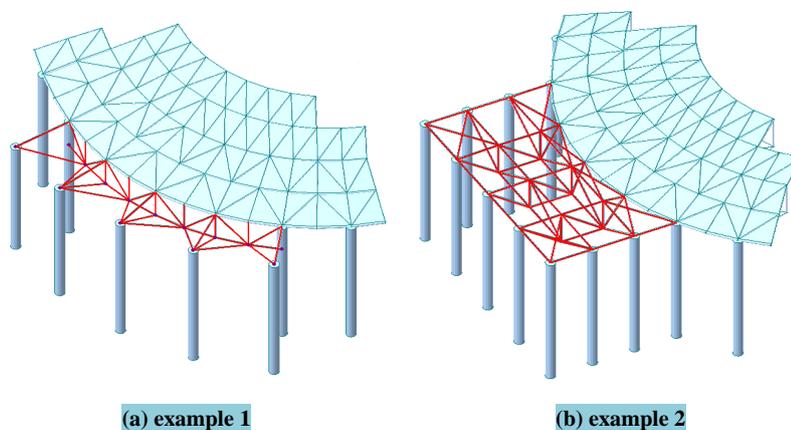


Fig. (8). Schematic diagram of local assistant supporting system.

4. PARAMETRIC ANALYSIS OF ASSISTANT SUPPORT PLAN

Besides the below structure provides a single circular concrete column as the supporting system, the missing columns' part of the structure can be replaced by the assistant supporting system. And this is able to support the suspended-dome with the external concrete column together. Depending on the actual situation, the local assistant supporting system can choose different forms. We have simulated two kinds of local assistant supporting systems as shown in Fig. (8), and were emphasized in red color.

4.1. The Theory of Equivalent Vertical Stiffness Based on Classical Beam Theory

Because of a variety of specific forms of auxiliary support system, when studying the applicability of this system, the mechanical properties of its own have not been considered, and only the vertical supporting stiffness provided by the suspended-dome's bearings will be analyzed. The assistant supporting system can be simplified as sketched in Fig. (9).

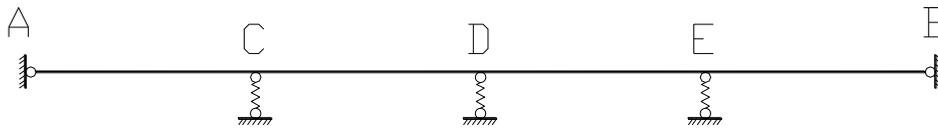


Fig. (9). Simplified schematic of supporting system.

In which A, B are the position of end columns and C, D, E are the position of the missing columns. The beam between A and B work as end columns' supporting effect. Where C, D, E, are spring bearings, representing the assistant supporting system. The vertical stiffness of node C, D, E is vertical stiffness provided by the assistant supporting system for suspended-dome, respectively.

However, because of various specific forms of local truss, the stiffness ratio of AB beam and CDE spring support cannot be determined. The stiffness of the spring bearing is also not necessarily the same. According to the most unfavorable conditions, this mean this beam is considered as hinged beam with supporting point. Assuming the beam length is \$l\$ and the distance between the target position and the bearing is \$x\$, by using the beam theory, the vertical stiffness of the target position in the bidirectional hinged beam can be calculated as:

$$K_l^x = \frac{3l \cdot EI}{(l-x)^2 x^2} \tag{1}$$

Given a simple truss cantilevered 8m, and its stiffness can be changed from 20000 to 80000kN/m. Assume 6 columns missed, and in the case of the assistant supporting system put forward formally. The vertical stiffness' ratio of the limit value to sever examples is shown in Fig. (10).

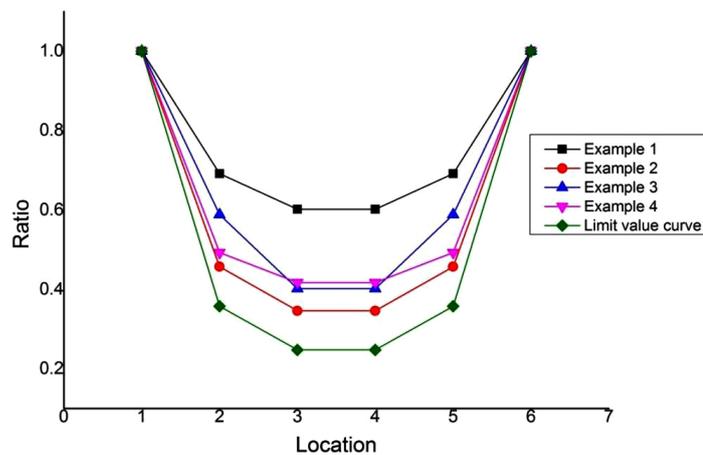


Fig. (10). Ratio of stiffness curves.

Obviously, it is safe and feasible to take equation (1) as the bearing point of the vertical stiffness limit curve.

The interval between columns is U , numbers of missing columns are n , and the equation of the vertical stiffness is shown in equation (2).

$$K_n^x = \frac{EI(n+1)}{U^3[(n-x+1)x]^2} \quad (2)$$

Due to the interval between columns are generally between 6 and 8 m, so this factor can be ignored. Define the following stiffness limit constant:

$$B = EI / U^3 \quad (3)$$

Substituting equations(3) in equations(2), the vertical stiffness of point x is formulated as follows:

$$K_n^x = \frac{(n+1)}{[(n-x+1)x]^2} B \quad (4)$$

4.2. Parametric Analysis of Equivalent Vertical Stiffness Theory

In order to highlight the contrast between the plan 1 and 2, the same parameters of plan 2 with plan1 is defined [12]. Obviously, if the value of B is too large, there will be little difference between the normal model and plan 2. If the value of B is too low, the assistant supporting system will continue to fall short of its stress. In this case, B has an upper and lower bound, to be chosen according to the stiffness of point x . Define the maximum value of vertical stiffness as 50000kN/m and the minimum value as 1000kN/m. Based on the interval [1000, 50000] kN/m, the upper and lower bound of B can be draw from above equations. In the span of 40m, 80m, and 120m, respectively, increase the number of missing columns and impose vertical spring constraint and horizontal hinge constraint at point x , furthermore, the upper and lower bound of B at different conditions can be obtain and the maximum numbers of missing columns for plan 2. Detailed parametric analysis plan is shown in Table 5.

Table 5. Parametric analysis plan.

Number of Missing Columns	Span, m		
	40	80	120
4	7200-129600	7200-86400	7200-86400
6	20571-205710	20571-205710	20571-205710
8	44444-266664	44444-266664	44444-266664

For span 40m, general correlation between maximum stress ratio of local member and vertical stiffness is not obvious, and the same to the number of missing columns. The reaction provided by the assistant supporting system is mainly related to the vertical stiffness of the most edge missing columns and show little consistent with the number of missing columns. The basic rule is that the lower the vertical stiffness of the most edge missing column is, the less the reaction is provided by the assistant supporting system. The maximum displacement of structure is mainly related to the number of missing columns. The major trend is that the larger the number of missing columns and the smaller the stiffness of the assistant supporting system, the larger displacement of the structure. In the case of 80m and 120m span, results of members of the assistant supporting truss, maximum stress ratio, reaction and maximum displacement, are the same as the 40m, respectively.

Maximum stress ratio, reaction and maximum displacement of the assistant supporting truss's members could show the change of structure's mechanical performance. Minimum limit vertical stiffness B and matching vertical stiffness are chosen to be shown in Table 6.

As shown in Figs. (11 and 12), through the comparison of three kinds of span displacement, when selecting an assistant supporting system with the same stiffness, reactions provided for suspended-dome have little difference at different span. However, the larger the span is, the smaller the change of displacement is. When the number of missing columns is the same, the smaller span of suspended-dome is more sensitive than the larger one. Therefore, a larger

stiffness of the assistant supporting system is required when design the structure.

Table 6. Minimum limit vertical stiffness B and corresponding vertical stiffness.

Span, m	40, 80, 120	
number of missing columns	minimum limit vertical stiffness B	Corresponding vertical stiffness, kN/m
4	43200	13500, 6000, 6000, 13500
6	123426	24000, 8640, 6000, 6000, 8640, 24000
8	266664	37500, 12245, 7407, 6000, 6000, 7407, 12245, 37500

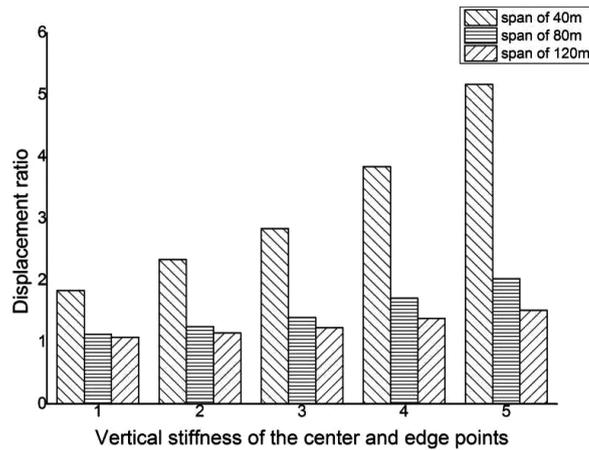


Fig. (11). Comparison of suspended-dome of missing 6 columns between different stiffness.

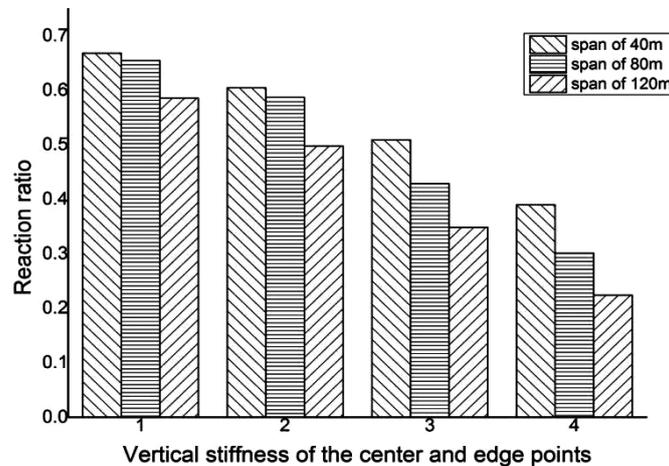


Fig. (12). Comparison of suspended-dome of missing 8 columns between different stiffness

4.3. Validation of Equivalent Vertical Stiffness Model

Define the stiffness of the most middle column of all missing columns by 6000kN/m. Considering safety, the stiffness provided by the assistant supporting system of the missing point should be greater than the limit value in Fig. (10). Given a suspended-dome as an example, which is 120m span and has 10 missing column. Applying the basic information into equations (3), the value of B can be shown as follows:

$$B = \frac{(5 \times 6)^2}{(10 + 1)} \times 6000 = 490910$$

Substituting B in equations (4) and the stiffness of all point can be calculated.

$$K_{10}^1 = K_{10}^{10} = \frac{(10+1)}{[10 \times 1]^2} \times 490910 \approx 54000 \text{ kN} / \text{m}$$

$$K_{10}^2 = K_{10}^9 = \frac{(10+1)}{[9 \times 2]^2} \times 490910 \approx 16667 \text{ kN} / \text{m}$$

$$K_{10}^3 = K_{10}^8 = \frac{(10+1)}{[8 \times 3]^2} \times 490910 \approx 9375 \text{ kN} / \text{m}$$

$$K_{10}^4 = K_{10}^7 = \frac{(10+1)}{[7 \times 4]^2} \times 490910 \approx 6888 \text{ kN} / \text{m}$$

$$K_{10}^5 = K_{10}^6 = \frac{(10+1)}{[6 \times 5]^2} \times 490910 \approx 6000 \text{ kN} / \text{m}$$

Based on the design variables calculated formerly, a suspended-dome is designed to corroborate the equivalent stiffness theory. Keep the size of suspended-dome members and the prestressing force, through calculation and analysis, the maximum stress ratio is 0.987 and the maximum displacement is 163mm which located in the seventh ring. The load factor matching to the instability point is 4.24 in double nonlinear analysis and complies with the requirements of the design code.

CONCLUSION

In this paper, two kinds of feasible plans were proposed for suspended-dome with big partial holes. Through establishing and analyzing parametric models, the suitable range of the two proposed plans has been obtained.

(1) For plan 1, when the number of missing columns is not more than 2, this problem can be solved by adjusting members of the suspended-dome. However, the smaller the span is and the more missing columns are, the greater the missing columns impact on the overall stability is, and the possibility of causing local vibration is also greater.

(2) For plan 2, the relationship between suspended-dome and stiffness of the assistant supporting system is that the smaller the vertical stiffness is, the larger the stress ratio is. Furthermore, the smaller the vertical stiffness of the most edge column is, the smaller the reaction provided by the assistant supporting system. Besides, the greater the number of missing columns is, the smaller the stiffness of the assistant supporting system and the greater the structural displacement.

AUTHOR CONTRIBUTIONS

NC and RR designed the study; NC and RC performed the experiments and analyzed the data; NC, RC and RR wrote the manuscript.

ETHICS APPROVAL AND CONSENT TO PARTICIPATE

Not applicable.

HUMAN AND ANIMAL RIGHTS

No Animals/Humans were used for studies that are base of this research.

CONSENT FOR PUBLICATION

Not applicable.

CONFLICT OF INTEREST

The authors confirm that this article content has no conflict of interest.

ACKNOWLEDGEMENTS

Declared none.

REFERENCES

- [1] M. Kawaguchi, M. Abe, and I. Tatemichi, "Design, tests and realization of suspen-dome system", *J. Int. Assoc. Shell Spat. Struct.*, vol. 40, no. 3, pp. 179-192, 1999.
- [2] S. Kitipornchai, W. Kang, and H.F. Lam, "Factors affecting the design and construction of Lamella suspen-dome systems", *J. Construct. Steel Res.*, vol. 61, no. 6, pp. 764-785, 2005.
[<http://dx.doi.org/10.1016/j.jcsr.2004.12.007>]
- [3] K. Mamoru, and A. Masaru, "Structural Tests on the "Suspen-dome" System", *IASS-ASCE Symposium*, 1994pp. 383-392
- [4] C.X. Wang, *MIDAS/Gen application examples tutorial and troubleshooting*. China Architecture and Building Press: Beijing, 2010.
- [5] X.W. Hou, *Answers to frequently asked questions in Midas Gen.*, China Architecture and Building Press: Beijing, 2014.
- [6] Z.H. Chen, and Y. Li, "Parameter analysis on stability of a suspen-dome", *Int. J. Space Structures*, vol. 20, no. 2, pp. 115-124, 2005.
[<http://dx.doi.org/10.1260/0266351054764218>]
- [7] Z.H. Chen, Y.J. Wu, and Y. Yin, "Formulation and application of multi-node sliding cable element for the analysis of Suspen-Dome structures", *Finite Elem. Anal. Des.*, vol. 46, no. 9, pp. 743-750, 2010.
[<http://dx.doi.org/10.1016/j.finel.2010.04.003>]
- [8] *GB 50011-2010 Code for Seismic Design of Buildings.*, China Architecture & Building Press: Beijing, 2010.
- [9] *JGJ7-2010 Technical Specification for Space Frame Structures.*, China Architecture & Building Press: Beijing, 2010.
- [10] *GB 50009-2012 Load Code for the Design of Building Structures.*, China Architecture & Building Press: Beijing, 2012.
- [11] *GB 50017-2003 Code for Design of Steel Structures.*, China Architecture & Building Press: Beijing, 2003.
- [12] Uri Kirsch., "Optimized prestressing by linear programming", *Int. J. Numer. Methods Eng.*, vol. 7, no. 2, pp. 125-136, 1973.
[<http://dx.doi.org/10.1002/nme.1620070204>]

© 2017 Jinghai et al.

This is an open access article distributed under the terms of the Creative Commons Attribution 4.0 International Public License (CC-BY 4.0), a copy of which is available at: <https://creativecommons.org/licenses/by/4.0/legalcode>. This license permits unrestricted use, distribution, and reproduction in any medium, provided the original author and source are credited.