

Thermo-Dynamics and Stress Characteristics on High Strength Hydraulic Concrete Material

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Abstract: High strength hydraulic concrete was introduced based on its thermo-dynamic and strain-stress characteristics. The compound exponential thermo-dynamic creep model was established. The temperature rise models were interpreted. The bisection constitutional model was deduced as the approach to express the strain-stress non-linearity of high strength hydraulic concrete. The key parameters on thermo-dynamic characteristics and strain-stress ones of high strength hydraulic concrete were offered. The models and parameters in this paper are useful for engineering application and theoretical research.

Keywords: High strength hydraulic concrete, non-linear model, thermo-dynamics.

1. INTRODUCTION

High strength and fully graded concrete has been the predominant material for Sino hydraulic structures. Therefore, these monstrous structures' working behaviors are controlled mainly by the mechanic characteristics of the massive concrete materials.

Unfortunately, the comprehensive study on concrete found in Sino-mainland was abated by the overwhelming construction work.

The target concrete material in this paper was produced in State Key Laboratory of Hydrosience and Hydraulic Engineering of Tsinghua University. All these parental materials including gravel, sand, cement and silica fume etc. were achieved in the construction site of South-Western China. The concrete's constituents' percentages are shown in detail in Fig. (1) where the values are mass percentages. The sand and gravel were taken from Jinshajiang River catchment where the main lithologic character is basalt layer; the range of gravel size is 5-20 mm and the relative compaction value of sand is 0.35 (M. Bravo and J. de Brito, 2012) [1]; the water used here is Northern China pure water which can be applied instantly for lives and the pH is 7.0; the moderate-heat cement powder is produced by South-Western China Xuanwei producer and the grade is 425 of Sino-mainland standard; the fly-ash powder is produced by Nantong Haichuang producer in Jiangsu Province and its application can depress effectively the hydration heat generation, especially during the early stage of concrete pouring procedure

(J. S. Ryou and Y. S. Lee, 2012) [2]; the producer for silica fume used here is Langtian Company of Sichuan Province in South-Western China and silica fume will increase the strength of concrete material (X. Y. WANG, 2013) [3]; the water reducing agent is the tripolycyanamide series produced by Xiangbang company in Jiangsu Province, and it is the indispensable addition here because it decreases the water supplement while the silica fume is used (M. Nili and V. Afroughsabet, 2012) [4]; the air entraining agent, which can densify the concrete, is colophony one produced by Qicheng company in Shanghai.

The high strength concrete were poured and modeled into normal samples for the creep loading test. The normal concrete samples' dimension (*i.e.*, length, width and height) are 100mm×100 mm×300 mm. C40 samples are the study targets in this paper where C_n designates the concrete strength grade in Sino-mainland and the unit for grade n is MPa.

2. THERMO-DYNAMIC CHARACTERISTICS ON HIGH STRENGTH HYDRAULIC CONCRETE

Especially, these hydraulic structures are often undermined by influential temperature creep deformation during the long, uncertain and sinister construction period (Q. WANG *et al.*, 2011; Z. W. LI *et al.*, 2012) [5, 6]. The temperature cracks in hydraulic concrete material and structures will incur hazardous damage in the development which may be an overwhelming disaster for the whole catchment. Some typical temperature cracks on the concrete surface from Sino hydraulic structures are shown in Fig. (2).

The key technique for temperature cracks control is the quantification on thermo-dynamic parameters and models of the high strength concrete. The key thermo-dynamic subjects on temperature cracks control of concrete material are the

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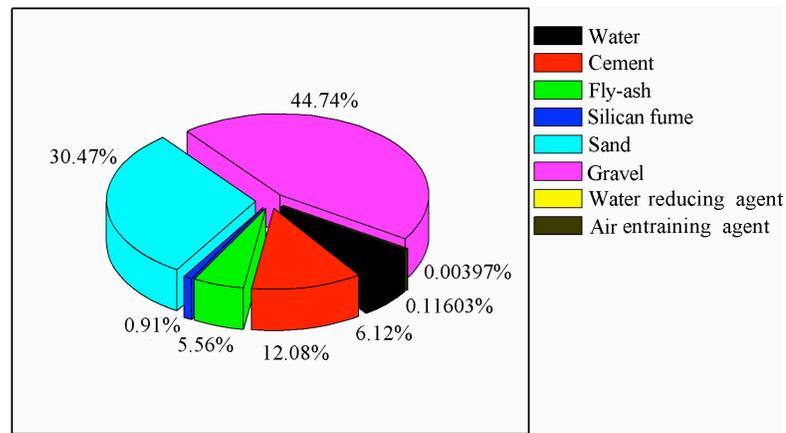


Fig. (1). High strength hydraulic concrete constituents (with mass percentages).



Fig. (2). Typical temperature cracks on the surface of hydraulic concrete structures.

thermo-dynamic creep models and temperature rise models. The generally acceptable thermo-dynamic creep model in Sino-hydraulic research is the compound exponential model. The merits of the compound exponential thermo-dynamic creep model are that it can unitize the thermo-dynamic simulation on the recoverable creep and unrecoverable creep. The compound exponential thermo-dynamic creep model can be expressed as Eq.1.

$$C(t, \tau) = \left[A_0 + A_1 \cdot \tau^{(-A_2)} \right] \cdot \left[1 - e^{-m_1(t-\tau)} \right] + \left[B_0 + B_1 \cdot \tau^{(-B_2)} \right] \cdot \left[1 - e^{-m_2(t-\tau)} \right] + D \cdot e^{(-m_3 \cdot \tau)} \cdot \left[1 - e^{-m_3(t-\tau)} \right] \quad (1)$$

where, $C(t, \tau)$ refers to the concrete's thermo-dynamic creep value; τ is the static age from the curing start point; t is the total age which includes the static age and loading period; $A_0, A_1, A_2, B_0, B_1, B_2, m_1$ and m_2 are the fitting parameters on concrete material's early stage recoverable creep; D and m_3 are the fitting parameters on concrete material's maturely unrecoverable creep; $t - \tau$ represents the loading duration increment.

Fig. (3) shows the normal concrete sample and creep loading test. As for the samples with the age of 7 d or 28 d, the eventual creep loading duration increment is 200 d that is the generally acceptable test standard on concrete creep test in

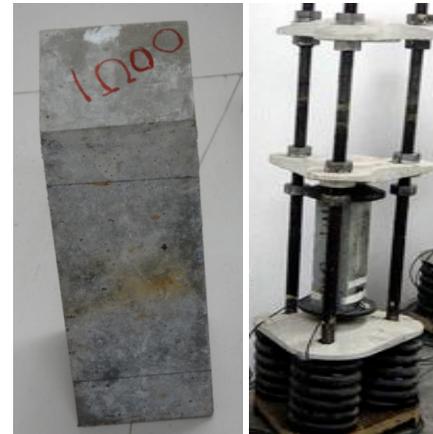


Fig. (3). Concrete creep loading test and concrete sample.

Sino-mainland. The instrument for concrete creep test is TXB- 50 system produced by Gangyuan company in Tianjin.

Concrete creep measured data are shown in Tables 1 and 2 where the numerical fitting results are also offered.

The numerically fitted curves on creep measured data of high strength hydraulic concrete are shown in Fig. (4).

It can be deduced from the creep loading test results on high strength concrete that the longer the curing age is, the lower the creep value is. According to most studies on high concrete materials, the curing activities are the key to

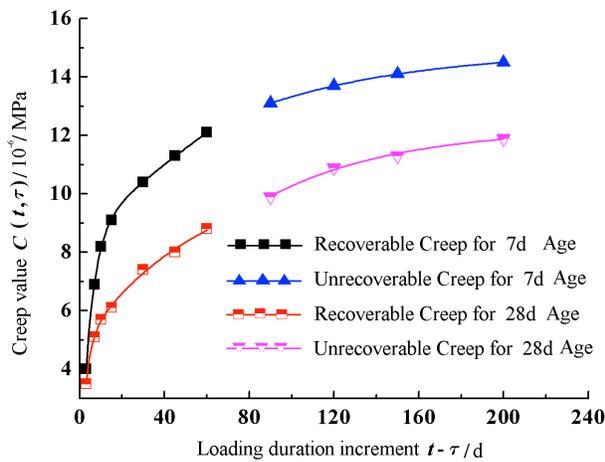


Fig. (4). Measured data and fitted curves from concrete creep loading tests on high strength hydraulic concrete.

reduce the creep value level. The frequent curing can also restrain the development of temperature cracks.

The hydration heat is the intrinsic character of concrete material and it is the main cause of temperature cracks in

hydraulic concrete material and structures. Therefore, the proper quantification for the hydration heat is the key technique for temperature cracks control on high strength hydraulic concrete. The temperature rise model can quantify numerically the magnitude of the hydration heat, especially during the dangerous period of concrete pouring. There live four temperature rise models in Sino hydraulic concrete thermo-dynamics, namely, power-exponential composite model, hyperbolic model, single-exponential model and double-exponential model. These models' functions are as follows:

$$T = T_0 \left(1 - e^{-at^b} \right) \tag{2}$$

$$T = \frac{28t}{t + 2.58} \tag{3}$$

$$T = T_0 \left(1 - e^{-at} \right) \tag{4}$$

$$T = T_c \left(1 - e^{-ct} \right) + T_1 \left(1 - e^{-dt} \right) \tag{5}$$

Table 1. Measured data and fitted creep parameters from concrete creep loading tests for sample C40, age: 7d.

Loading Duration Increment for Recoverable Creep/d	Recoverable Creep Value/10 ⁻⁶ /MPa	Fitted Parameters		Loading Duration Increment for Unrecoverable Creep/d	Unrecoverable Creep Value/10 ⁻⁶ /MPa	Fitted Parameters	
		<i>A₀</i>				<i>D</i>	
3	4	<i>A₀</i>	1.99	90	13.1	<i>D</i>	17.31437
7	6.9	<i>A₁</i>	19.81	120	13.7	<i>m₃</i>	0.02545
10	8.2	<i>A₂</i>	0.703	150	14.1		
15	9.1	<i>B₀</i>	3.97	200	14.5		
30	10.4	<i>B₁</i>	11.89				
45	11.3	<i>B₂</i>	1.495				
60	12.1	<i>m₁</i>	0.0892				
		<i>m₂</i>	0.10801				

Table 2. Measured data and fitted creep parameters from concrete creep loading tests for sample C40, age: 28d.

Loading Duration Increment for Recoverable Creep/d	Recoverable Creep Value/10 ⁻⁶ /MPa	Fitted Parameters		Loading Duration Increment for Unrecoverable Creep/d	Unrecoverable Creep Value/10 ⁻⁶ /MPa	Fitted Parameters	
		<i>A₀</i>				<i>D</i>	
3	3.5	<i>A₀</i>	2	90	9.9	<i>D</i>	20
7	5.1	<i>A₁</i>	18	120	10.9	<i>m₃</i>	0.01813
10	5.7	<i>A₂</i>	0.55	150	11.3		
15	6.1	<i>B₀</i>	3	200	11.9		
30	7.4	<i>B₁</i>	10				
45	8	<i>B₂</i>	0.8				
60	8.8	<i>m₁</i>	0.06				
		<i>m₂</i>	0.2				

where, T is the temperature rise variable; T_0 is the ultimate temperature rise value; T_e is the early stage temperature rise value; T_1 is the mature temperature rise value; a, b, c and d are the test parameters. As for high strength concrete C40, $T_0 = 26.4^\circ\text{C}$, $T_e = 5^\circ\text{C}$, $T_1 = 25^\circ\text{C}$, $a = 0.252$, $b = 0.883$, $c = 0.005$, $d = 0.252$.

These models' comparison is given in Fig. (5). According to Fig. (5), it can be deduced that the temperature rise values computed from those models converge at mature stage and the obvious discrepancy just lives at the early stage of hydration heat development.

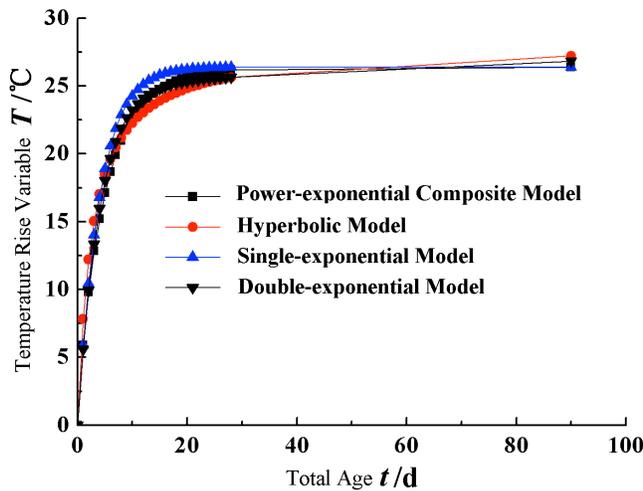


Fig. (5). Temperature rise models.

However, the fatal danger of thermo-dynamic fracture is often incurred at the early pouring stage of massively high strength concrete. Hence, the fly-ash powder is the indispensable constituent of the high strength concrete. It can reduce the hydration heat at the early stage of concrete pouring procedure (D. G. Snelson and J. M. Kinuthia, 2010; D. Krizan and B. Zivanovic, 1997) [7, 8].

3. STRESS CHARACTERISTICS OF HIGH STRENGTH HYDRAULIC CONCRETE

Stress characteristics analysis is also beneficial for deep insight into high strength hydraulic concrete research. Stress overloading is the main cause of hydraulic concrete material's failure (G. Habert *et al.*, 2012; S. A. kumar and A. Veeraragavan, 2011) [9, 10].

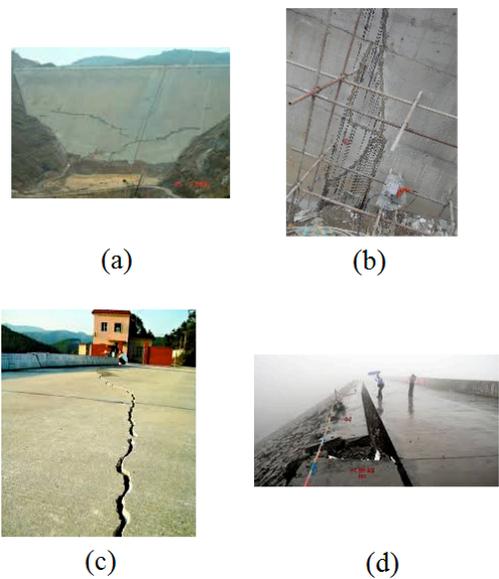


Fig. (6). Engineering cases on concrete cracks.

Stress development behaviors, whatever under static conditions or dynamic ones, control the reliability of most hydraulic structures (Fig. 6 where, (a) are the penetrated cracks in concrete gravity dam due to seismic stress development; (b) are the aged cracks on the surface of concrete arch dam due to static stress accumulation; (c) are the extending cracks in the shoulder of concrete face dam due to static stress accumulation; (d) are the eroded cracks in the crest of concrete face dam due to hydrodynamic stress development).

How to express the stress development behaviors of high strength hydraulic concrete? Strain-stress constitution on concrete plays the primary role in translating material's reliability and mechanical behavior (A. Turatsinze and M. Garros, 2008; A. Nazari and S. Riahi, 2011) [11, 12]. It is also the bases for other advanced technical models. Multi-axial compression loading tests were applied for the target high strength hydraulic concrete material. Multi-axial compression loading tests include uniaxial and biaxial compression loading tests.

The tensiometers are the key supplementary units in these tests to help get the complete strain-stress constitutional curves (Fig. 7 where (a) is instron multi-axial loading system; (b) is uniaxial compression loading case and the red

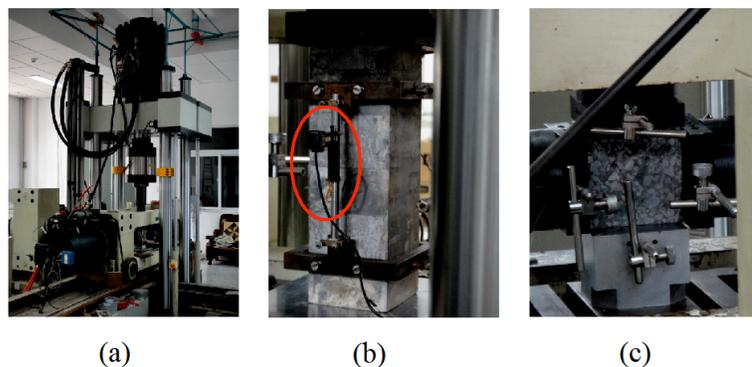


Fig. (7). Multi-axial compression loading tests.

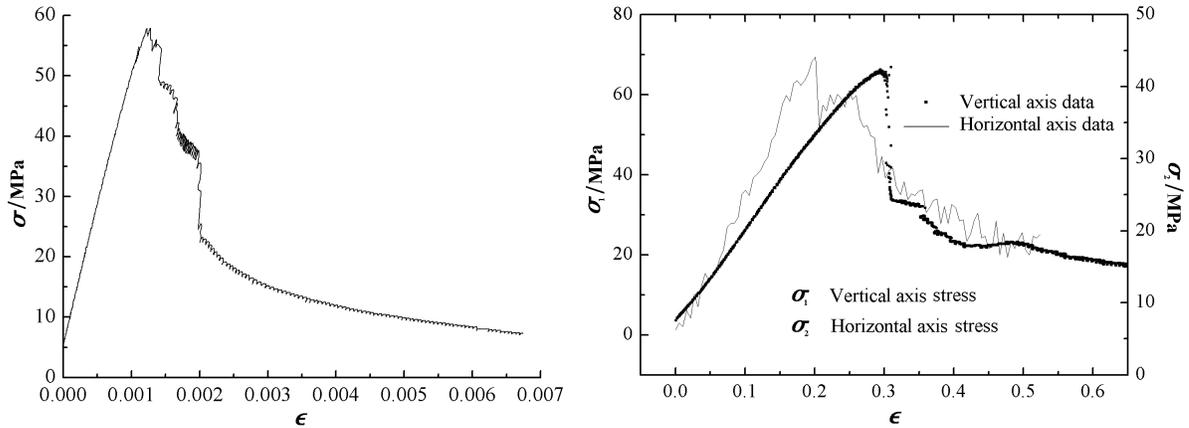


Fig. (8). Strain-stress curves from multi-axial compression loading tests.

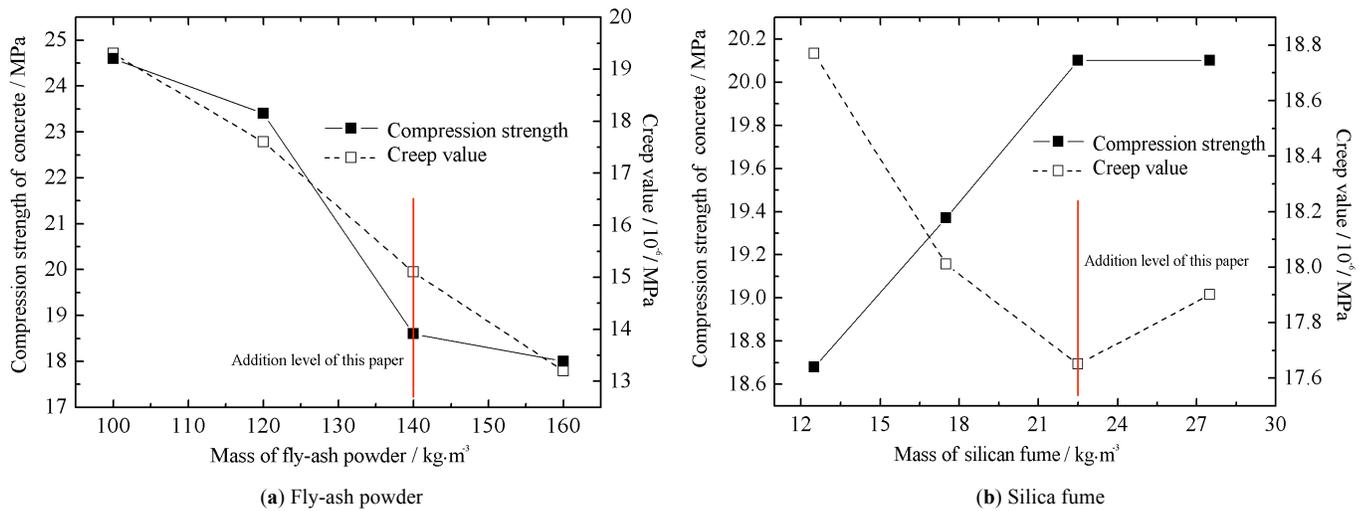


Fig. (9). Concrete characteristics sensitivity to constituents' addition.

Table 3. Reference values for concrete constitution parameters.

ϵ_f	α_1	α_2	β_1	β_2	η	E_0 / MPa
1.23×10^{-3}	1.57×10^{-1}	8.43×10^{-1}	5.39	1.66	3.47	5.58×10^4

circle area is the tensiometer; (c) is biaxial compression loading case.).

The strain-stress curves from both uniaxial and biaxial compression loading cases show that the mechanical behavior of the target high strength hydraulic concrete complies with non-linear constitutional model. The values of ultimate strength of uniaxial and biaxial compression loading cases are different ones. The vertical axial ultimate strength of biaxial compression loading case is 10 MPa averagely higher than the one of uniaxial compression loading case. However, the horizontal axial ultimate strength of biaxial compression loading case is 14 MPa averagely lower than the one of uniaxial compression loading case. Fig. (8) clearly shows all these characteristics.

In Sino-mainland, the generally accepted strain-stress constitution model for concrete is the bisection non-linear model which can be expressed as the Eq. 6.

$$\sigma = \begin{cases} \left[1 - \alpha_1 \left(\frac{\epsilon}{\epsilon_f} \right)^{\beta_1} \right] E_0 \epsilon & 0 \leq \epsilon \leq \epsilon_f \\ \left[\frac{\alpha_2}{\eta \left(\frac{\epsilon}{\epsilon_f} - 1 \right)^{\beta_2} + \frac{\epsilon}{\epsilon_f}} \right] E_0 \epsilon & \epsilon > \epsilon_f \end{cases} \quad (6)$$

where, σ and ϵ are the stress and strain variables respectively; ϵ_f is the ultimate strain of concrete under compression loading case; α_1 , α_2 and β_1 are the concrete material parameters that indicate the continuousness of the bisection non-linear model at ϵ_f ; β_2 and η are the shape parameters that indicate the geometrical characteristics of the non-linear strain-stress curves; E_0 is the nominal modulus. The reference values of these parameters are shown in Table 3.

4. DISCUSSION

The study on characteristics sensitivity is valuable for ascertaining the effects of constituents' application in high strength hydraulic concrete. The study is based on internal addition method, namely, the cement was replaced by agents such as, fly-ash powder and silica fume, directly and variably. Fly-ash powder and silica fume are the main targets to be researched for their effects (Fig. 9). The compression strength of the concrete material will go lower with the increase of fly-ash addition level. By contrast, its creep values will run lower, too. The compression strength of the concrete material will grow with the increase of silica fume addition level. However, the increasing effect tends to converge. By contrast, its creep values will reach on peak with the increase of silica fume addition level.

CONCLUSION

High strength hydraulic concrete constituents were introduced. Thermo-dynamic characteristics can be expressed with creep models and temperature rise models. They are helpful for the quantification of the concrete material's temperature cracks hazard. The bisection non-linear model can explain the intrinsic non-linearity of strain-stress constitution of high strength hydraulic concrete. The proper addition of fly-ash powder and silica fume is the effective approach to improve the strength of hydraulic concrete.

CONFLICT OF INTEREST

The authors confirm that this article content has no conflict of interest.

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